

Stormwater Management Strategy

5483 & 5495 Princes Highway
Traralgon Victoria

Client

Stable Property Services

Issued

19/09/2025



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 Project Number: 2101947

Surveying
 Asset Recording
 Civil Engineering
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Water Resource Engineering
 Strata Certification (NSW)
 Town Planning
 Urban Design
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 Project Management

Revision Table

REV	DESCRIPTION	DATE	AUTHORISED
A	Drainage Strategy	25/03/2022	[REDACTED]
B	Drainage Strategy Updated ISP	13/12/2022	[REDACTED]
C	Drainage Strategy Updated ISP (Commercial Lot)	18/05/2023	[REDACTED]
D	Drainage Strategy (Removed ISP)	20/03/2025	[REDACTED]
E	Drainage Strategy (Response to WGCMA)	19/09/2025	[REDACTED]

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1. INTRODUCTION

Beveridge Williams has been commissioned by Stable Property Services to prepare a Stormwater Management Strategy (SWMS) for a proposed residential development site located at 5483 & 5495 Princes Highway, Traralgon, Victoria. The total site area is approximately 58.13 ha, and it is proposed to develop the land into residential lots and a neighbourhood activity centre (commercial area). The development is also proposed to include open space and drainage reserves.

This SWMS is intended to provide sufficient evidence that the stormwater discharges from the proposed development can meet stormwater Best Practice Environmental Management Guidelines (BPEMG) and shall be to the satisfaction of West Gippsland Catchment Management Authority (WGCMA), Latrobe City Council, and other relevant authorities.

1.1. Site Overview

The subject site is located approximately 4.5 km southwest of the Traralgon town centre and is bounded by Princes Highway to the south, Bradford Drive to the southeast, Regan Road to the northeast, and existing rural residential developments to the west and north. Additionally, within the site boundary there is an existing dwelling, farm buildings, and dams as shown in **Figure 1**.

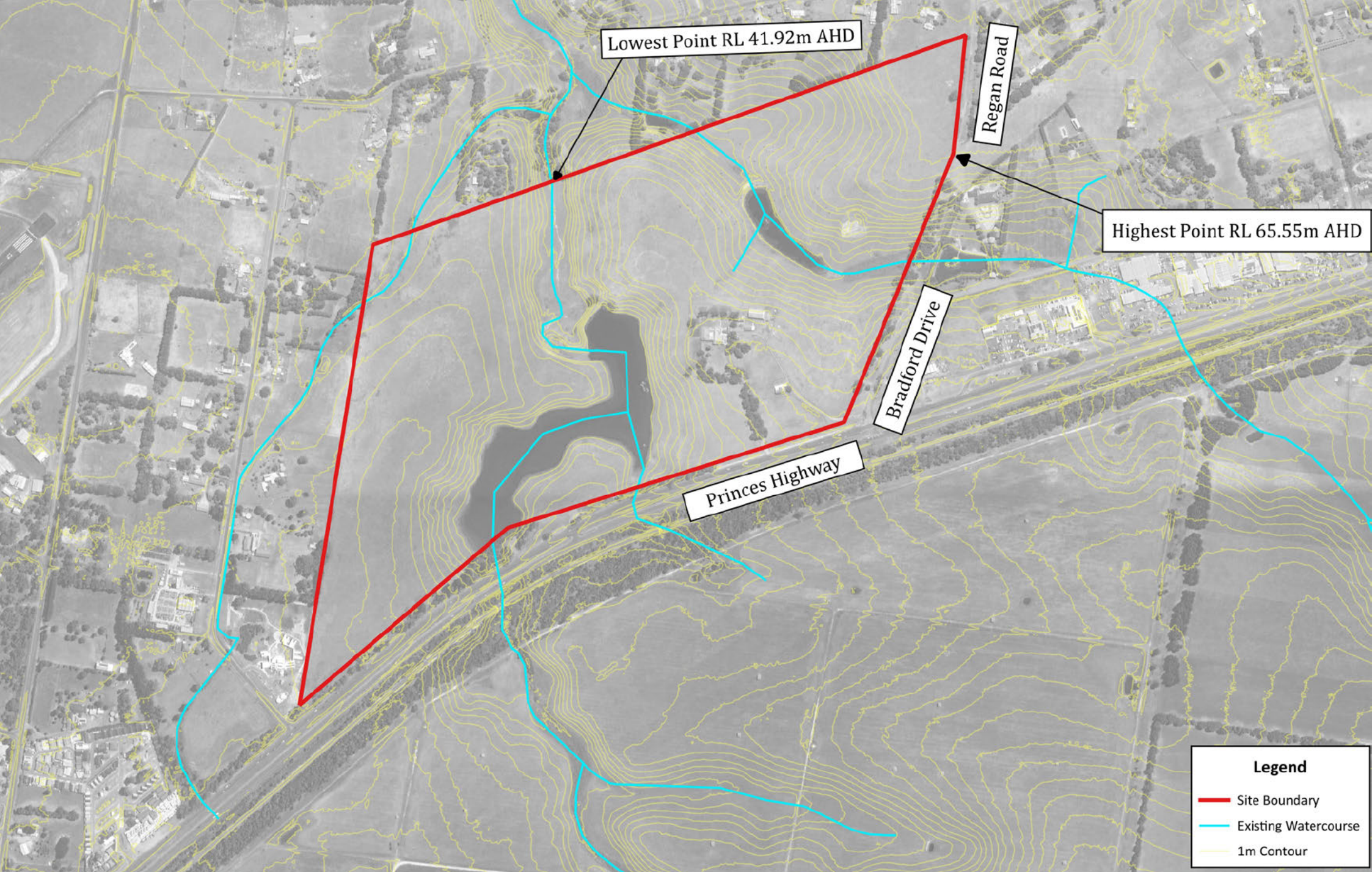
1.2. Project objectives

This study aims to provide a conceptual Stormwater Management Strategy (SWMS) to support the rezoning of land from Farming Zone to General Residential 3 Zone and Commercial 1 Zone.

The investigations undertaken for this report has identified on-site stormwater management solutions based upon a proposed development typology, and has identified a concept drainage strategy for the site which will:

- Ensure the management of stormwater quantity by attenuating developed peak 1% AEP runoff to existing levels if required
- Achieve best practice stormwater quality management targets
- Ensure the site has the capacity to provide the required stormwater quality and quantity management assets

The SWMS for the site is founded on best practice principles and achieves compliance with the requirements of the Water Act 1989, ARR 19, and Best-Practice Environmental Management Guidelines (CSIRO, 1999) and WSUD Engineering Procedures.



Lowest Point RL 41.92m AHD

Regan Road

Highest Point RL 65.55m AHD

Bradford Drive

Princes Highway

Legend

- Site Boundary
- Existing Watercourse
- 1m Contour

2. EXISTING CONDITIONS

2.1. Topography

The site generally slopes in two directions, from south to north and from east to north. A small portion of the area on the west slopes towards the north direction. The highest point of the site is located on the east (RL 65.55 m AHD) and the lowest point is located at the north (RL 41.92 m AHD). The average slope of the site is approximately 1 in 30.

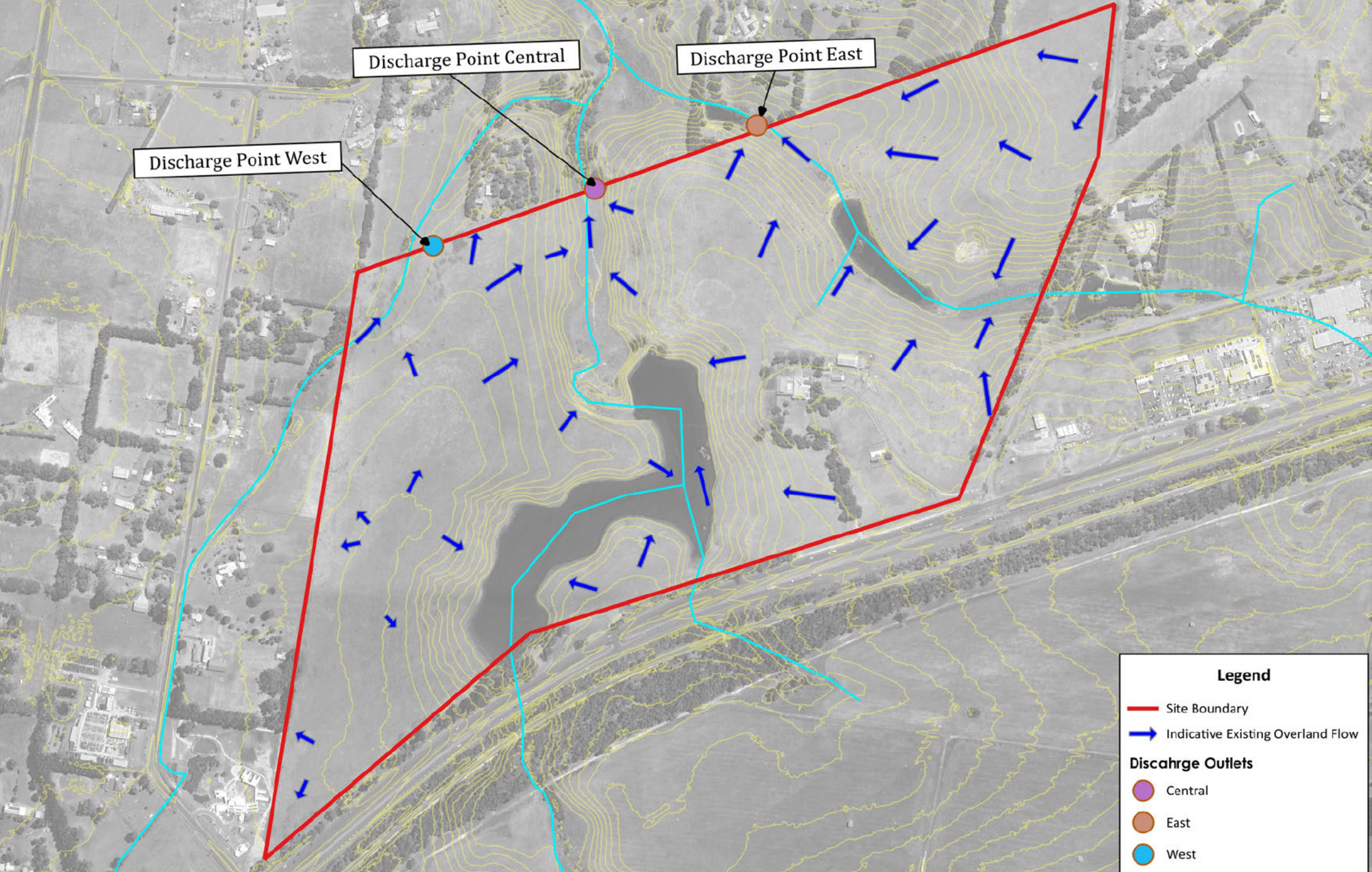
A plan showing the site topography is shown in **Figure 1**.

2.2. Surface Water and Drainage

In existing conditions, surface water flows from the high point on the east and west towards to north. These flow paths are shown in **Figure 2**.

Surface water from external catchment of approximately 1,732 ha from the south and east of the site currently drains into the site. Outside the southern boundary of the site, there is a designated farmland area while towards the eastern boundary there are mixed use land such as residential, vacant land and commercial areas.

The existing topography as shown in **Figure 2** shows the three existing discharge points in the proposed development site (west, central and east). There is an isolated catchment towards the west which naturally drains north. Flows from all discharge points follow an existing overland flow path downstream and converge into the existing Boyds Creek approximately 100 metres north of the site.



Discharge Point Central

Discharge Point East

Discharge Point West

Legend

- Site Boundary
- ➔ Indicative Existing Overland Flow

Discharge Outlets

- Central
- East
- West

3. DESIGN INTENT

3.1. Proposed Development

The proposed subdivision includes a residential area of approximately 58.13 ha/520 residential lots, open space areas, and includes a commercial space of approximately 2.14 hectares. The internal road network for the proposed subdivision will be designed in accordance with their function and have a cross section that are of sufficient width to facilitate the provision of on street parking and pedestrian paths.

3.2. Design Criteria

This report is intended to investigate the feasibility of managing stormwater within the proposed development site. The proposed development will require further investigation after a development layout has been provided. Provided that the overall concept is not significantly altered, the development layout is unlikely to impact the water quantity and quality management strategy presented in this report. It shall be noted however, that the increase in impervious areas within the proposed development are likely to impact the outcomes and calculations undertaken in this report.

It is appropriate for details of the drainage concept design to be reviewed and optimised at the detailed design stage. This is typical practice for development, with detailed design occurring once a planning permit has been granted. Should there be any concerns regarding details of the proposed works, these can be addressed through appropriate conditions within a planning permit.

3.3. Proposed Stormwater Management Strategy

This stormwater management strategy has been proposed to follow the existing natural features of the site for both stormwater quantity and quality management. The aim of this SWMS is to assess surface water impact for downstream property owners and users as result of this development.

Stormwater quality management is proposed to be provided by using Rainwater tanks on individual lots, Gross Pollutant Traps (GPT), and Wetlands, while for stormwater quantity management, it has been determined that flow restriction do not need to be implemented due to the low difference in flow due to the proposed development.

It is a Victorian government requirement that the quality of stormwater runoff from proposed developments meets the Urban Stormwater Best Practice Environmental Management Guidelines (BPEMG) which are required under clause 56 of the Victorian Planning Provisions (VPP). The water quality treatment will be designed to meet Best Practice Environmental Management Guidelines (BPEMG).

4. STORMWATER QUANTITY MANAGEMENT

Details of the stormwater quantity management for the proposed site is discussed in the following sections.

4.1. Hydrology

The hydrologic analysis of the 1% AEP flows for the proposed development site was undertaken using RORB modelling to determine the pre-developed and post-developed flows. The external and pre-development catchment plan is presented in **Figures 3** and **4** respectively.

The internal catchments for the pre-development conditions have been delineated using survey data, contour information and aerial imaging. The internal post-development conditions catchments were delineated assuming a typical development with 70% impervious areas. The post-developed catchment is shown in **Figure 5**.

4.2. RORB Hydrologic Modelling

Hydrological analysis for the proposed development was undertaken using RORB Runoff Routing Program Modelling in accordance with the Australia Rainfall and Runoff 2019 guidelines to determine the design flows for the pre-developed and post-developed scenarios. Hydrological model inputs were obtained from the ARR Data Hub using coordinates Latitude -38.204 and Longitude 146.491.

4.2.1. Temporal Patterns

As the total area of the modelled catchment is approximately 17.9 km², temporal patterns were used to simulate the storm duration for input into the hydraulic model.

4.2.2. Areal Reduction Factors

The Areal Reduction Factors (ARFs) were calculated based on the Australian Rainfall and Runoff (ARR) (Book 2, Chapter 4.3) considering the catchment size of 17.9 km².

4.2.3. Rainfall Temporal Pattern

The design rainfall inputs were based on the ARR Data hub temporal patterns stratified by the rainfall events. The IFD depths for catchment centroid were obtained from the Bureau of Meteorology (BOM) website.

4.2.4. Loss Approach

The initial loss and continuous loss values were extracted from the ARR data hub for the catchment (22 mm and 43 mm/hr respectively) and used for the initial hydrologic model setup. The storm losses values that were adopted as final are given in **Table 1**.

4.2.5. Routing Parameters

The m value was left unchanged as the default of 0.8.

A k_c value was initially selected from the RORB Default Equation for the model setup. This value was then calibrated to the 1% AEP discharge obtained from the Regional Flood Frequency Estimation Model (RFFE). Values obtained from the RFFE were then compared to the Vic (MAR > 800 mm) Equation (7.6.15) ARR(BkVII), RORB Default Equation and Victoria Data (Pearse et al, 2002). Values obtained from each method are given in **Table 2**.

The k_c value adopted for the pre-developed scenario was obtained using the Victoria Data (Pearse et al, 2002) due to its closeness to the RFFE 1% AEP flows result (Refer to **Table 2**). The final k_c value adopted was then obtained by adjusting the initial and continuous loss obtained from the ARR Data Hub (see **Table 2**).

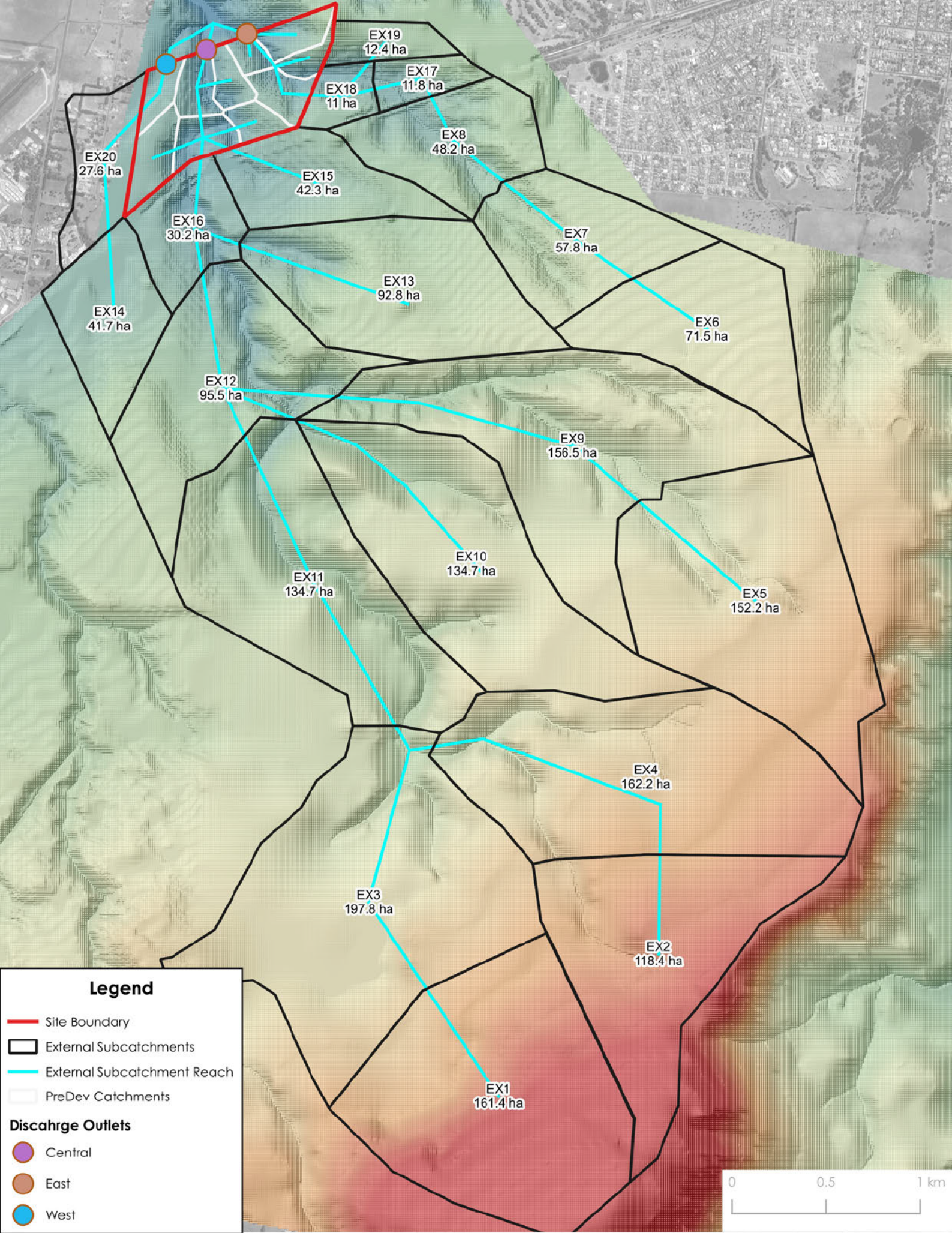
In the post-developed scenario, the k_c value was determined using the k_c to d_{av} ratio from the obtained pre-development parameters. Details of the RORB routing parameters used for the pre-development and post development scenarios are given in **Table 2**.

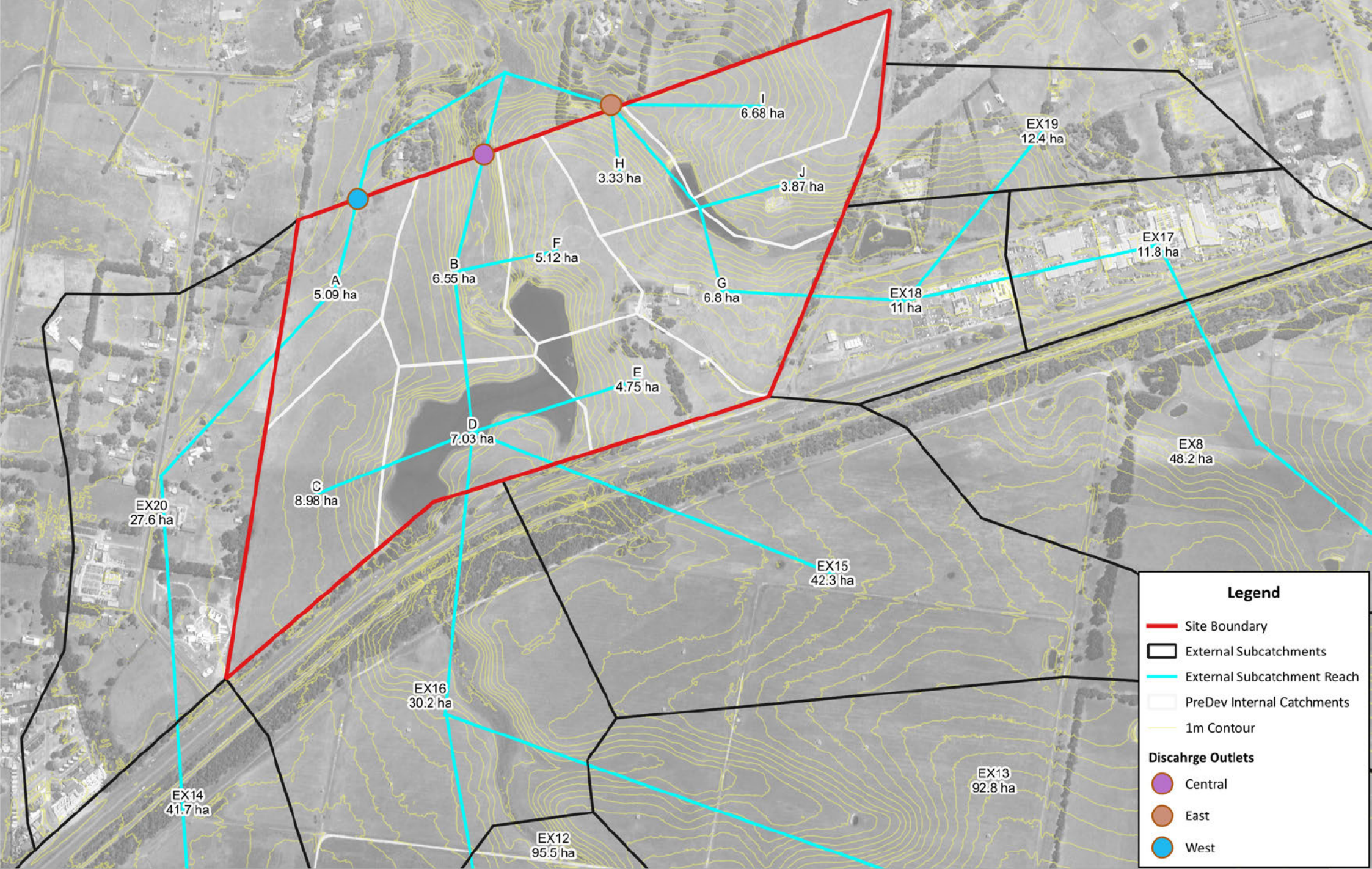
4.2.6. 75% Preburst Depths

Additionally, the 75th percentile pre-burst depths were obtained from the Data Hub for each available storm duration. The extract obtained from the Data Hub is shown in **Appendix A**. These values are then used to determine the loss factor for each storm duration. The loss factors were then calculated for each storm duration using the equation below.

$$\text{Loss factor} = \frac{\text{Initial loss} - 75^{\text{th}} \text{Percentile Pre Burst}}{\text{Initial loss}}$$

Using the above equation, loss factors were obtained for the following storm durations. The loss factors are then included in the RORB modelling and the results are analysed to provide the flow results in **Appendix B**. The modelling includes associated external sub-catchments areas to the south and east of the site (Refer to **Figure 3**).







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Plan Name: Figure 5: Post-Development Catchment Plan- subject site only (Not to Scale)
 Address: 5483 & 5495 Princes Highway, Traralgon, Victoria
 Client: Stable Property Services

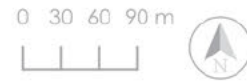


Table 1: Hydrological Parameters and Considerations (RORB Model)

DESIGN PARAMETER	REQUIREMENT/ASSUMPTION	COMMENT
Software package	RORB	
Rainfall Estimation Procedures	ARR 2019	
IFD Data	ARR 2016 Design Rainfall	Bureau of Meteorology
Losses	Initial Losses (IL) = 22 mm Continuous Losses (CL) = 4.3 mm/h m = 0.8	Australian Rainfall & Runoff Data Hub-Results
Catchments	Existing Catchments mapped from 1m gridded ALS LiDAR and detailed survey. Proposed Catchments adjusted to account for post development earthworks grading	Geoscience Australia, Beveridge Williams Based on Civil Design
Catchment Area	17.9 km ²	Development site and External Catchments
RFFE Result	Discharge = 45 m ³ /s	1% AEP Discharge
Kc Parameters for Pre-Developed Scenario	RORB Default Equation = 9.31 Vic (Mar > 800 mm) = 9.41 Victoria Data (Pearse et al, 2002) = 5.23	Corresponding Kc results of for each calibration method
1% AEP Pre-Development Calibration Methods	RORB Default Equation = 27.44 m ³ /s Vic (Mar > 800 mm) = 27.15 m ³ /s Victoria Data (Pearse et al, 2002) = 50.64 m ³ /s	Pre-Developed flow calibration flow results for the different methods used.
Regional equation	$K_c = 1.25 \times d_{av}$	Victoria data [Pearse et al. (2002)]
Calibrated 1% AEP Pre-Development Flow	Victoria Data (Pearse et al, 2002) = 48.43 m ³ /s	RORB Results
Calibrated Storm Losses	Initial Losses (IL) = 25 mm Continuous Losses (CL) = 5.7 mm/h	RORB Results
k _c Values	4.81 4.87 (Ex South). 1.94 (Ex East) and 0.87 (Site)	Pre-developed Post-developed
Average Distance (d _{av})	3.86 km 3.9 km	Pre-developed Post-developed
Design Events	1% AEP	Standard frequencies to and including the 1% AEP
Post-Developed Impervious %	70 %	

4.3. Modelled Events and Hydrology Results

4.3.1. 1% AEP Peak Flow Rates

The results of 1% AEP pre-development and post-development peak flows from the critical locations from the RORB modelling are shown in **Table 2**. Locations of external inflows and discharge points are considered as critical locations, as shown in **Figure 4**. The Ensemble method was used in RORB to determine the critical durations.

Details of the RORB results for the pre-development and post development scenarios are given in **Appendix C**.

Table 2: 1% AEP Flow Results at Critical Locations

Location	1% AEP Pre Dev	1% AEP Post Dev	1% AEP Difference
External South	35.04 m ³ /s at 3 hr	-	-
Discharge Point West	4.24 m ³ /s at 1 hr	5.54 m ³ /s at 1 hr	+1.30
Discharge Point Central	34.78 m ³ /s at 3 hr	38.53 m ³ /s at 3 hr	+3.75
External East	9.19 m ³ /s at 3 hr	-	-
Discharge Point East	9.70 m ³ /s at 3 hr	10.95 m ³ /s at 3 hr	+1.25
Overall Outlet	44.39 m³/s at 3 hr	49.36 m³/s at 3 hr	+4.97

Table 2 indicates flow attenuation differences pre and post development are minimal.

It is suggested that flow attenuation measures are not required for the development due to the difference between pre and post developed flows being only 4.97 m³/s out of 49.36 m³/s which is a difference of only 8%. In the overall scheme of events and based on today's climatic conditions, there is a case for flow attenuation to be disregarded.

4.4. Sub-surface Drainage (20% AEP)

The subsurface drainage will be sized to convey the 20% AEP storm event flows. These flows will be conveyed via the subsurface drainage network system and will be discharged from the site at two points of discharge as shown in **Figure 4**. The subsurface drainage network can be detailed in the later stages of design once a development plan has been provided.

4.5. Subject Site Overland Flow

Overland flows from the subject site will be conveyed through road networks and drainage reserves to the legal points of discharge. The internal roads for the development, and associated lot finished surface levels, will be designed to ensure that the Q_{gap} flows are conveyed through the site within the safe hydraulic capacity of road floodway.

5. STORMWATER QUALITY MANAGEMENT

The stormwater quality analysis for this study was undertaken using MUSIC 6 software to demonstrate that the stormwater management system proposed for the development will meet the stormwater treatment requirements in accordance with Water Sensitive Urban Design Guidelines prior to discharge downstream. The treatment requirements in the guideline provides target pollutant removal rates of 80% Total Suspended Solids (TSS), 45% Total Nitrogen (TN), 45% Total Phosphorus (TP) and 90% Gross Pollutants (GP).

5.1. MUSIC Model Development

A MUSIC model was prepared to reflect the proposed development as shown on **Figure 6** below. The model considers 2 kL rainwater tanks (RWT) on each residential lot with most of the catchments within the site ultimately discharging into three Wetlands around the site via Gross Pollutant Traps (GPT).

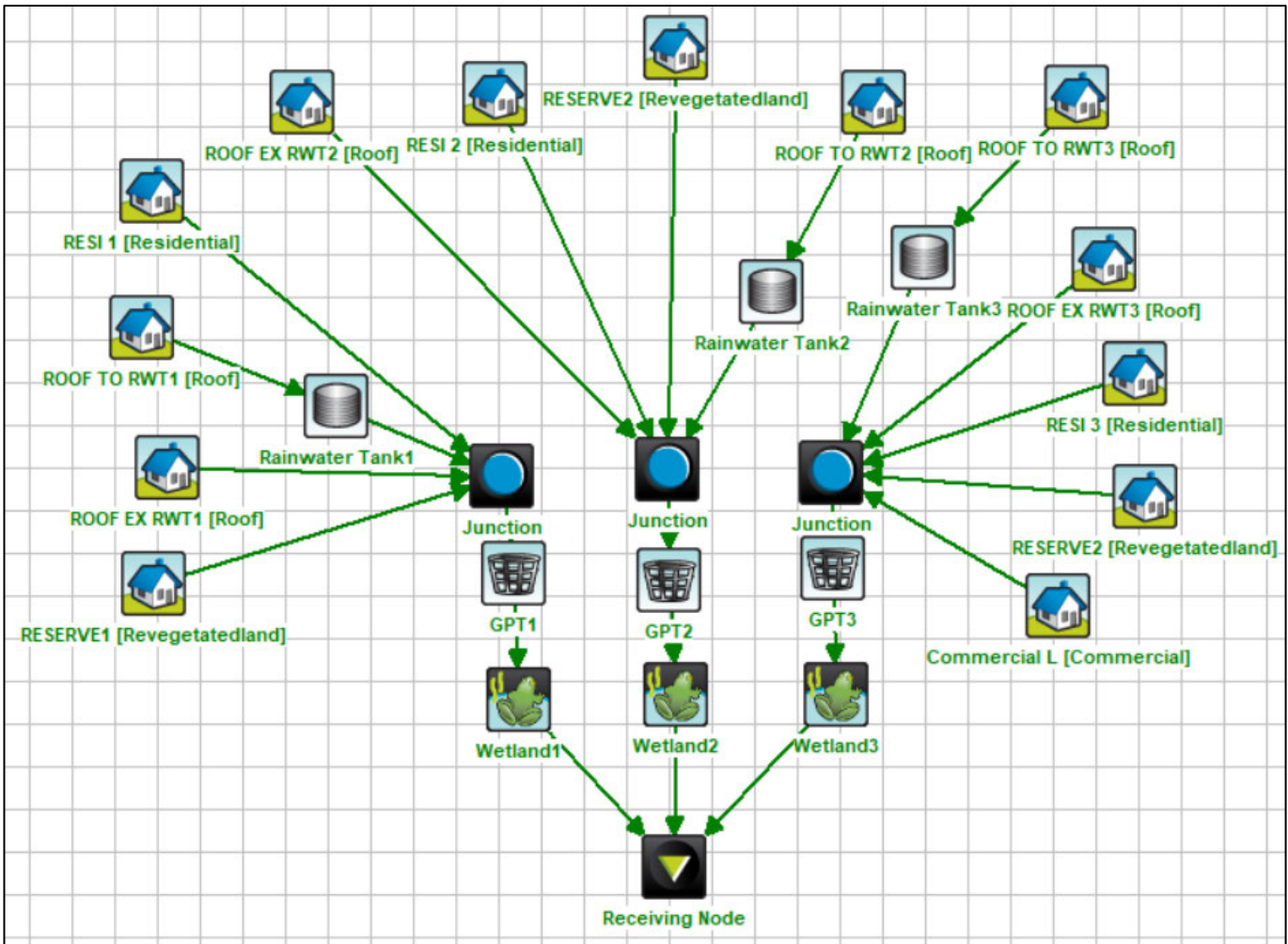


Figure 6: Music Layout

5.2. Modelling Parameters and Assumptions

The following inputs were implemented in the MUSIC model:

- 6-minute rainfall data from Traralgon L.v.w.& S.b. 085170 was adopted
- The following source node parameters were provided for various catchment areas:

Table 3: MUSIC Soil Parameters

PARAMETER	VALUE
Rainfall Threshold (mm)	1
Soil Storage Capacity (mm)	120
Initial Storage (%)	25
Field Capacity (mm)	50
Infiltration Capacity Coefficient A	200
Infiltration Capacity Exponent B	1
Initial Depth (mm)	10
Daily Recharge Rate (%)	25
Daily Baseflow Rate (%)	5
Daily Deep Seepage Rate (%)	0

Table 4: MUSIC Pollutant Generation Parameters

PARAMETER	VALUE
TSS Baseflow Mean (Log mg/L)	1.1
TSS Baseflow Std Dev (Log mg/L)	0.17
TSS Stormflow Mean (Log mg/L)	2.2
TSS Stormflow Std Dev (Log mg/L)	0.32
TP Baseflow Mean (Log mg/L)	-0.82
TP Baseflow Std Dev (Log mg/L)	0.19
TP Stormflow Mean (Log mg/L)	-0.45
TP Stormflow Std Dev (Log mg/L)	0.25
TN Baseflow Mean (Log mg/L)	0.32
TN Baseflow Std Dev (Log mg/L)	0.12
TN Stormflow Mean (Log mg/L)	0.42
TN Stormflow Std Dev (Log mg/L)	0.19

Table 5: Rainwater Tank Parameters (per tank)

PARAMETER	VALUE
Low Flow By-pass (m ³ /s)	0.00
High Flow By-pass (m ³ /s)	0.17
Volume below overflow pipe(kL)	2.00
Depth above overflow (m)	0.20
Surface Area (m ²)	1.4
Initial Volume (kL)	2.00
Overflow pipe diameter (mm)	90
Re-Use	
Daily Demand Properties (kL/day)	0.22

Table 6: Gross Pollutant Trap

PARAMETER	VALUE
Low Flow By-pass (m ³ /s)	0.00
High Flow By-pass (m ³ /s)	0.009
Concentration Based Capture Efficiency	
Input	Output
0.00	0.00
15.00	0.00
Flow Based Capture Efficiency	
Input	Output
0.00	0.00
1.00	100.00

Table 7: Wetland Input Parameters

PARAMETER	VALUE
Low Flow By-pass (m ³ /s)	0.00
High Flow By-pass (m ³ /s)	100.00
Wetland 1	
Surface Area (m ²)	5,000
Extended Detention Depth (m)	0.35
Permanent Pool Volume (m ³)	16,000
Initial Pool Volume (m ³)	16,000
Exfiltration Rate (mm/hr)	0.00
Evaporative Loss as% of PET	125
Equivalent Pipe Diameter (mm)	70
Overflow Weir Width (m)	3.0
Notional Detention Time (hrs)	72
Wetland 2	
Surface Area (m ²)	4,000

Extended Detention Depth (m)	0.35
Permanent Pool Volume (m ³)	1,300
Initial Pool Volume (m ³)	1,300
Exfiltration Rate (mm/hr)	0.00
Evaporative Loss as% of PET	125
Equivalent Pipe Diameter (mm)	60
Overflow Weir Width (m)	3.0
Notional Detention Time (hrs)	78

Wetland 3

Surface Area (m ²)	6,000
Extended Detention Depth (m)	0.35
Permanent Pool Volume (m ³)	2,000
Initial Pool Volume (m ³)	2,000
Exfiltration Rate (mm/hr)	0.00
Evaporative Loss as% of PET	125
Equivalent Pipe Diameter (mm)	76
Overflow Weir Width (m)	3
Notional Detention Time (hrs)	73

5.3. MUSIC Model Results

Model results are presented in **Table 8** and demonstrate that all pollutant load reduction targets have been achieved through the proposed treatment system.

Table 8: MUSIC Result Summary

	Best Practice Reduction Targets (%)	Achieved Reduction Results (%)
Total Suspended Solids (kg/yr)	80	80.0
Total Phosphorus (kg/yr)	45	64.9
Total Nitrogen (kg/yr)	45	52.6
Gross Pollutant (kg/yr)	70	100

As shown above in **Table 8**, the proposed MUSIC layout meets the best practice BPEMG standard for the proposed development.

6. CONCLUSION

Beveridge Williams have developed a Concept Stormwater Management Strategy in support of the rezoning of land from Farming Zone to General Residential 3 Zone and Commercial 1 Zone. Investigations undertaken have determined the following requirements for the development.

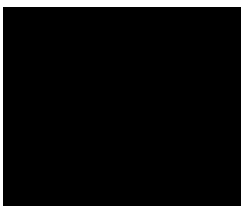
- As the difference between pre and post developed flows are only 4.97 m³/s out of 49.36 m³/s which is a difference of 8% in the overall scheme of events, there is a case for no flow attenuation to be implemented.
- Three Wetland systems with surface area ranging from 4000 m² to 6000 m² with upstream GPT's, and Rainwater Tanks provided on each residential lot will provide stormwater quality management for the proposed development to ensure pollutant removal targets are met prior to discharge from the site.

In summary, the study shows that stormwater management requirements can be achieved within the site effectively through flow attenuation and water quality systems for a residential development to be feasible. The hydrologic modelling determined that flow attenuation is not required for the proposed development.

It is recommended that further investigations to be undertaken in the detail design phase to design the proposed drainage assets incorporating the effects of climate change.

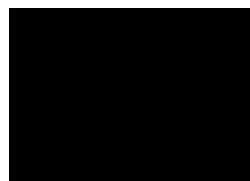
BEVERIDGE WILLIAMS & CO PTY LTD

Prepared by



Water Resources Engineer

Reviewed by



Senior Water Resources Engineer

Approved for issue by



Principal Town Planner

APPENDIX A: ARR DATAHUB 75TH PREBURST DEPTH INFORMATION

75% Preburst Depths

Values are of the format depth (ratio) with depth in mm

min (h)\AEP(%)	50	20	10	5	2	1
60 (1.0)	9.4 (0.569)	13.5 (0.575)	16.3 (0.566)	18.9 (0.553)	13.4 (0.320)	9.3 (0.193)
90 (1.5)	12.4 (0.656)	14.0 (0.524)	15.1 (0.464)	16.2 (0.418)	14.8 (0.313)	13.7 (0.253)
120 (2.0)	9.4 (0.454)	12.3 (0.421)	14.2 (0.400)	16.0 (0.382)	16.7 (0.326)	17.2 (0.292)
180 (3.0)	11.0 (0.464)	14.2 (0.429)	16.3 (0.407)	18.3 (0.388)	19.5 (0.340)	20.4 (0.311)
360 (6.0)	5.5 (0.183)	10.5 (0.257)	13.8 (0.282)	17.0 (0.296)	21.3 (0.304)	24.5 (0.305)
720 (12.0)	3.2 (0.086)	8.1 (0.159)	11.3 (0.186)	14.4 (0.202)	17.3 (0.199)	19.4 (0.194)
1080 (18.0)	3.9 (0.091)	6.7 (0.115)	8.5 (0.123)	10.3 (0.126)	14.0 (0.141)	16.8 (0.147)
1440 (24.0)	2.8 (0.060)	5.5 (0.087)	7.3 (0.096)	9.0 (0.101)	10.1 (0.093)	11.0 (0.087)
2160 (36.0)	0.0 (0.000)	1.0 (0.014)	1.6 (0.019)	2.2 (0.022)	3.4 (0.028)	4.3 (0.030)
2880 (48.0)	0.0 (0.000)	0.3 (0.003)	0.4 (0.005)	0.6 (0.006)	1.3 (0.010)	1.8 (0.012)
4320 (72.0)	0.0 (0.000)	0.0 (0.000)	0.0 (0.000)	0.0 (0.000)	0.2 (0.001)	0.3 (0.002)

1% AEP Loss Factor

STORM DURATION (h)	75TH PERCENTILE PRE-BURST DEPTH (mm)	INITIAL LOSS (mm)	INITIAL LOSS – 75TH PRE-BURST (mm)	LOSS FACTOR
1	9.3	22	12.7	0.58
1.5	13.7		8.3	0.38
2	17.2		4.8	0.22
3	20.4		1.6	0.07
6	24.5		-2.5	-0.11
12	19.4		2.6	0.12
18	16.8		5.2	0.24
24	11.0		11.0	0.50
36	4.3		17.7	0.80
48	1.8		20.2	0.92
72	0.3		21.7	0.99

APPENDIX B: RORB RESULTS

